

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
	4th				
	not on AIC Bill.				
	Record measurement				
	N/W - Const. of H.L. <sup>R.C.C.</sup> Bridge in Patahi				
	Jhanda chaur to Sugapipar Rd.				
	in Kodariya village under				
	State scheme 4515 (NABARD)				
	Block - Patahi.				
	Agency - M/s Kumar Const. Company.				
	C/O - Surendra Kumar.				
	Chandmari, Motihari, Champaran.				
	A/c. No. - 20 SBD/2023-24.				
	Const. cost - Rs. 6,00,39,769 = 60				
	Maint. cost - Rs. 9,54,538 = 60				

	Date of start - 14.12.2023.				
	Date of completion - 8.1.24.				
	(1) Reinforcement in abut. cap -				
	(103) main 25 $\Phi$ - 1 x 98 x 7.95 m				
	@ 3.85 Ky/m = 2999.54 Ky.				
	Bottom 25 $\Phi$ - 1 x 40 x 15.15 m				
	@ 3.85 Ky/m = 2333.10 Ky.				
	Chairs - 16 $\Phi$ - 1 x 15 nos x 6.00 m.				
	@ 1.58 Ky/m = 142.20 Ky.				
	(102) 20 $\Phi$ top layer - Bottom bar -				
	1 x 33 nos x 15.15 m				
	@ 2.47 Ky/m = 1234.88 Ky.				
	(101) 20 $\Phi$ top layer top bar -				
	20 $\Phi$ - 1 x 81 x 7.95 m.				
	@ 2.47 Ky/m = 1590.56 Ky.				

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(106) Binder -					
	16 $\Phi$	2x2	6x13	15m	
			@ 1.58 kg/m		= 498.65 kg.
(106A) Binder - 16 $\Phi$ -					
	2x2	6x5	9.55m		
			@ 1.58 kg/m		= 225.62 kg.
					T-9024.55 kg.
					Say, 9025 kg.
(2) P.V. R. C.C. M <sub>25</sub> in abut					
(A1) cap - do - do - E/I.					
	1	12.30	5.10	1.80	= 12.91 m <sup>3</sup>
<del>24<math>\Phi</math> 24</del>					
<del>J.F.</del>					
(3) Reinforcement in Abut (A2)					
cap - do - do - E/I.					
(103) main 25 $\Phi$ - 1x98x7.95m.					
			@ 3.85 kg/m		= 2999.54 kg.
Bottom 25 $\Phi$ - 1x46x15.15m					
			@ 3.85 kg/m		= 2333.10 kg.
Chair - 16 $\Phi$ - 1x15x6.10m					
			@ 1.58 kg/m		= 142.20 kg.
(102) 20 $\Phi$ top layer, Bottom -					
	1	33	15.15m		
			@ 2.47 kg/m		= 1234.88 kg.
(101) Top layer top bay -					
	20 $\Phi$	1x81	7.95m		
			@ 2.47 kg/m		= 1590.56 kg.

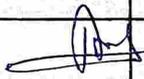
Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(106) Binder-16 $\Phi$ -					
		2x2x6x	13.15M		
			@1.58ky/m	=498.65ky.	
(106A) Binder 16 $\Phi$ -					
		2x2x6x	5.95M		
			@1.58ky/m	=225.62ky.	
			T-3024.55ky.		
			Say, 3.025MT.		
(4) P.V. R.C.C. M <sub>25</sub> in abut (A <sub>2</sub> )					
comp - do-do-EI.					
		1x	12.30x5.10x1.80	=112.91m <sup>3</sup>	
<del>Adj.</del> 28.394 J.E.			<del>13.15</del> AC		
(5) Reinforcement in Abut					
(A <sub>1</sub> ) shaft - do-do-EI.					
Earth face @ nose portion -					
(110) 25 $\Phi$ - 1x86x5.43M					
			@3.85ky/m	=1797.87ky.	
(110A) 20 $\Phi$ - 1x68x5.43M					
			@2.47ky/m	=912.02ky.	
(111) 12 $\Phi$ - shaft distribution -					
		1x2x14x	10.55M		
			@0.89ky/m	=262.91ky.	
(124) Vertical bar R/W -					
16 $\Phi$ - 1x116x			5.97M		
			@1.58ky/m	=1094.18ky.	
(125) Distribution bar - R/W -					
		1x44x	5.84M		
			@0.89ky/m	=228.69ky.	

Continuation



Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(125) Distribution R/W -					
	120	1 x 44	5.84 m		
			@ 0.89 k4/m		228.69 k4.
(126) 12 $\Phi$ -	1	44	5.84 m		
			@ 0.89 k4/m		228.69 k4.
(127) 16 $\Phi$ -	2	4	5.97 m		
			@ 1.58 k4/m		75.46 k4.
(112) 10 $\Phi$ -	1	30.2	1.30 m		
			@ 0.62 k4/m		243.41 k4.
			T -		4843.23 k4.
			Say,		4.843 MT.
(8) P/W: R.C.C. M30 in Abut (A <sub>2</sub> )					
Shaft to R/W - do - E/J.					

Shaft	- 1 x	8.45	x 1.20 x	1.833 =	18.59 m <sup>3</sup>
R/W	- 1 x	2 x 350	x $\frac{0.65 \times 0.30}{2}$	x 1.833 =	6.09 m <sup>3</sup>
					24.68 m <sup>3</sup>

					
4.4.24.					
S.E.					

(9) Reinforcement in abut. (A)					
Hammer - do - E/J.					

(113) 16 $\Phi$ - stirrups -					
	1	3 x 57	3.20 m		
			@ 1.58 k4/m		432.29 k4.

(114) Distribution - 16 $\Phi$ -					
	1	16	8.35 m		
			@ 1.85 k4/m		105.545 k4.

(115) Shaft cap (Hammer Head) -					
Distribution outer -					

Continuation



Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(11) Reinforcement in Abut.					
(A2) shaft cap (Masonry Head)					
— do — do — B/I					
(113) 16 $\Phi$ - 315 MPa -					
1x3x57x3.20m					
@ 1.58 k $\frac{kg}{m}$ = 432.29 k $\frac{kg}{m}$					
(114) Distribution bar -					
16 $\Phi$ - 1x16x8.35m					
@ 1.58 k $\frac{kg}{m}$ = 105.545 k $\frac{kg}{m}$					
(115) Shaft cap -					
Dirt wall outer - 12 $\Phi$ - 1x57x3.67m					
@ 0.89 k $\frac{kg}{m}$ = 186.18 k $\frac{kg}{m}$					
Dirt wall inner -					
12 $\Phi$ - 1x57x3.37m					
@ 0.89 k $\frac{kg}{m}$ = 170.96 k $\frac{kg}{m}$					
Padestal (M <sub>1</sub> ) - 16 $\Phi$ -					
1x3x6x2.37m					
@ 1.58 k $\frac{kg}{m}$ = 67.405 k $\frac{kg}{m}$					
(M <sub>2</sub> ) - 16 $\Phi$ - 1x3x6x2.37m					
@ 1.58 k $\frac{kg}{m}$ = 67.405 k $\frac{kg}{m}$					
(M <sub>3</sub> ) - 8 $\Phi$ - 1x2x3x18.84m = 45.215 k $\frac{kg}{m}$					
(M <sub>7</sub> ) - 12 $\Phi$ - 1x2x12x3.98m = 85.01 k $\frac{kg}{m}$					
(M <sub>8</sub> ) - 8 $\Phi$ - 1x2x2x23.51m					
@ 0.40 k $\frac{kg}{m}$ = 37.62 k $\frac{kg}{m}$					
					1197.63 k $\frac{kg}{m}$
					Say, 1198 MT.

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(12) P.W. R.C.C. M30 abut (A <sub>2</sub> )					
Shaff cap - do - E.I.					8.957m <sup>3</sup>
1 $\phi$ x 8.45 x 1.325 x 0.80					71.656m <sup>3</sup>
<del>11.4.24</del>					
J.E					
(13) Reinforcement in pier					
(P <sub>1</sub> ) Cap - do - do - E.I.					
(203) Main bar 1st layer -					
25 $\phi$ - 69 x 7.95 m					
@ 3.85 kg/m					= 2111.92 kg.
(204) 2nd layer -					
25 $\phi$ - 41 x 11.55 m					
@ 3.85 kg/m					= 1823.17 kg.
(205) Chair - 16 $\phi$ - 12 x 6.00 m					
@ 5.8 kg/m					= 113.76 kg.
TOP layer -					
(202) - 16 $\phi$ - 41 x 11.55 m					
@ 1.58 kg/m					= 748.209 kg.
TOP layer (201) - 16 $\phi$ - 69 x 7.95 m					
@ 1.58 kg/m					= 866.71 kg.
(206) Distribution -					
16 $\phi$ - 2 x 6 x 9.55 m					
@ 1.58 kg/m					= 181.07 kg.
(206A) Distribution -					
16 $\phi$ - 2 x 6 x 5.95 m					
@ 1.58 kg/m					= 119.81 kg.
					CO. 34-59 57.649 kg.

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
					B.F. 877-5957.649 kg
(207) Shaft reinforcement -					
25 $\Phi$ - 1x40 x 6.53 m.					
					@ 3.85 kg/m = 1005.62 kg
(208) Binder - 16 $\Phi$ - 10 x 5.66 m					
					@ 1.58 kg/m = 85.43 kg
					T-7052.699 kg
					Sq, 7.053 MT.
(14) P.V. R.C.C. M35 in pier					
(P <sub>1</sub> ) cap - do - E.I.F.					
1 x 8.70 x 5.10 x 1.80 = 79.866 m <sup>3</sup>					
<del>14.4.21</del> S.E.					
(15) Reinforcement in pier (P <sub>2</sub> )					
cap - do - E.I.F.					
(203) main bar Ist. layer -					
25 $\Phi$ - 69 x 7.95 m					
					@ 3.85 kg/m = 2111.99 kg
(204) 2nd layer -					
25 $\Phi$ - 41 x 11.55 m					
					@ 3.85 kg/m = 1823.17 kg
(205) chair - 16 $\Phi$ - 12 x 6.10 m.					
					@ 1.58 kg/m = 113.76 kg
(202) TOP layer - 16 $\Phi$ - 41 x 11.55 m					
					@ 1.58 kg/m = 748.209 kg
TOP bar (201) - 16 $\Phi$ - 69 x 7.95 m					
					@ 1.58 kg/m = 866.71 kg

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(206) Distribution bar -					
	16 $\phi$	- 2x6	9.55 m		
		@ 1.58 kg/m			= 181.07 kg.
(206A) Binder - 16 $\phi$ - 2x6x5.95m					
		@ 1.58 kg/m			= 112.81 kg.
(207) shaft reinforcement -					
	25 $\phi$	- 1x40	6.53 m		
		@ 3.85 kg/m			= 1005.62 kg.
(208) Binder - 16 $\phi$ - 10 x 5.66 m					
		@ 1.58 kg/m			= 29.43 kg.
					T - 7052.699 kg.
					Say - 7.053 MT.
(16) P.V. R.C.C. Mass in pier (P2)					
cap - do - do - E.I.I.					
	1	8.70 x 5.10	1.80		= 79.866 m <sup>3</sup>
<i>J.E.</i>					
17.4.24					
J.E.					
(17) Reinforcement in pier (P3)					
cap - do - do - E.I.I.					
(203) min bar 1st Layer					
	25 $\phi$	- 69	7.95 m		
		@ 3.85 kg/m			= 2111.92 kg.
(204) 2nd. layer -					
	25 $\phi$	- 41	11.55 m		
		@ 3.85 kg/m			= 1823.17 kg.

Continuation



Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(20) P.V. R.C.C. M35 in pier					
Shattz - do - E.I.					
					$1 \times 0.7857 \times (1.80)^2 \times 1.45 = 3.69 m^3$
<del>Am</del>					
25.4.24.					
J.E.					
(21) P.V. R.C.C. M35 in pier					
Shattz - do - do - E.I.					
					$1 \times 0.7857 \times (1.80)^2 \times 1.45 = 3.69 m^3$
<del>Am</del>					
27.4.24.					
J.E.					

Abstract of cost.					
(1/28)	const. of embankment with				
	approved material - do - E.I.				
	Qty. with T.M.B.P. NO. - (144)				
		$600.4 m^3 @ 211.35/m^3 =$	$RS. 126810 =$		$00$
(2/30)	P.V. enclosing R.C. G.N.P.				
	H.P. 1600 mm dia. - do - E.I.				
	Qty. with T.M.B.P. NO. - (144)				
		$52.50 m @ 814.8 =$	$RS. 427799 =$		$00$
(3/29)	const. of G. S.B. G.I. -				
	- do - do - E.I.				
	Qty. with T.M.B.P. No. - (145)				
		$3010 m^3 @ 4026 =$	$RS. 120795 =$		$00$
(4/2)	const. of service rel. -				
	- do - do - E.I.				

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
STY. VICK. T.M.B.P. NO. - (45)					
68.48 M. @ 4378 = 01/MT-RB-29					39806 = ∞
(5/1) E/W. in exca. in form					
- do - do - E.I.F.					
STY. VICK. T.M.B.P. NO. - (45)					
642.79 M <sup>3</sup> @ 205 = 54/M <sup>3</sup>					132119 = ∞
(6/8) S/F @ placing H.Y.S.D. bar					
- do - do - E.I.F.					
STY. VICK. T.M.B.P. NO. - (45)					
81.032 @ 77941 = 70/MT-RB-6315772 = ∞					
(7/3) P.V. Steel lining 6mm thick for pile					
- do - E.I.F.					
STY. VICK. T.M.B.P. NO. - (45)					
17.18 MT. @ 1066716 = 13/MT-RB-2046817 = ∞					
(8/4) P.V. Board cost in situ					
R.C.C. M35 grade - do - E.I.F.					
STY. VICK. T.M.B.P. NO. - (45)					
750.00 MT. @ 18536 = 11/MT-RB-13902083.00					
(9/8) Reinforcement in Abut. cap					
- do - do - E.I.F.					
STY. VICK. T.M.B.P. NO. - (49) - 9.025 MT					
" " " - (50) - 9.025 "					
" " " - (56) - 7.053 "					
" " " - (57) - 7.053 "					
" " " - (58) - 7.053 "					
					39.209 MT.
					@ 77941 = 70/MT-RB-3056016 = ∞

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(8/9) P.V. M30 R.C.C. in sub- struc - do - E.I.					
Sty. width F.M.B.P. NO - (51) - 24.68 m <sup>3</sup>					
" " " - (53) - 24.68 "					
" " " - (53) - 8.957 "					
" " " - (55) - 8.957 "					
" " " - (58) - 3.69 "					
" " " - (59) - 3.69 "					
" " " - (59) - 3.69 "					
					78.344 m <sup>3</sup>

@ 9537 = 60/m<sup>3</sup> RS - 7,47,214 = 00

(9/13) - Reinforcement in Abut  
Shaft or Pier Shaft - do - E.I.

Sty. width F.M.B.P. NO - (51) - 4.843 MT.					
" " " - (52) - 4.843 MT.					
" " " - (53) - 1.198 "					
" " " - (54) - 1.198 "					
					12.082 MT.
					@ 78163 = 57/m <sup>3</sup> RS - 9,44,372 = 00

(10/7) P.V. R.C.C. M35 in Abut  
Shaft or Pier Shaft -  
- do - E.I.

Sty. width F.M.B.P. NO - (49) - 112.91 m <sup>3</sup>					
" " " - (50) - 112.91 "					
" " " - (56) - 79.866 "					
" " " - (57) - 79.866 "					
" " " - (58) - 79.866 "					
					465.418 m <sup>3</sup>
					@ 9215 = 51/m <sup>3</sup> RS - 42,89,064 = 00

Continuation

C.O. RS - 3,24,08,667 = 00

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	No.	L.	B.	D.	
			B.F-RS	32408667=00	
Add- P.S.T. - 18%			- RS	5833560=00	
Add- L. Cess - 1%			- RS	324087=00	
Add- S. Fee			- RS	192981=00	
				RS: 3,87,59,295=00	
Less - 0.11%			- RS	42635=00	
				RS: 38716660=00	
Less provision payment					
ticketing m.B.P.NO - (B)			- RS	30,00,000=00	
" " " " - (43)			- RS	23946146=00	
				RS: 1,17,70,514=00	
27.4.24 J.E				RS: 974959	
				R. 10795555	

Continuation