

JAGDISHPUR UTTARWARI TOLA TO CHAKRA
in (Manigachhi Block) (MMGSY SC)
Schedule XLV-Form No. 134

Executive Engineer
B.W.D. (I) Division
Darbhanga - 2

_____ DIVISION

Manigachhi _____ SUB-DIVISION

Aq. No: → 07/SBD/2020-21

Measurement Book

Sri VPENDRA YADAV.

No. 860

Name of Work—
 Situation of Work—
 Agency by which work is executed—
 Date of Measurement—
 No. and date of agreement

(These four lines should be repeated at the commencement of the measurement relating to each work)

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
Name of work - constn. with Maintnce of work from jagdishpur Uttarwari to a to chakka in Naingachhi block.					
Name of Agency - Upendra yadav					
Vill. P.O - Kauripur,					
P.S. - Pandaul (Madhubani).					
Agmt. No. - 07/SBD/2020-21					
(MMSY-SC).					
Agmt. value - 01,98,34,593/-					
Date of start work - 20.6.2020.					
Time of compl work - 19.6.2021					
Situation of work - In progress.					
Work done -					
① Settg out of ref. & work's					
Bench-marks ——— of B.M.					
2/10-8					
② constn. of ref. pillars/burjees					
100/10-8					
③ clearing & grading of Ref. Area -					
$20 \times 30M \times 3.50M (ar) = 2100.00m^2$					
$20 \times 30M \times 3.50M (ar) = 2100.00m^2$					
$20 \times 30M \times 3.50M (ar) = 2100.00m^2$					
$19 \times 30M \times 3.50M (ar) = 1995.00m^2$					
$\rightarrow 8295.00$					
$\rightarrow 0.83 Hct.$					

Continuation

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(4) Constn. of embankment with					
mate. from broom grow pits					
chainage - (M)	Area - (M ²)	M. Area - (M ²)	Dist - (M)	Qty. - (M ³)	
250 -	1.096 -				
300 -	3.198	2.147	50	107.35	
350 -	4.092	3.645	50	182.26	
400 -	3.234	3.663	50	183.15	
450 -	2.526	2.880	50	144.00	
500 -	3.815	3.171	50	158.55	
550 -	4.373	4.094	50	204.70	
600 -	2.582	3.478	50	173.90	
650 -	2.530	2.556	50	127.80	
700 -	3.286	2.908	50	145.40	

750 -	1.878	1.052	50	52.85	
800 -	1.456	1.642	50	82.10	
850 -	1.660	1.558	50	77.90	
900 -	0.865	1.262	50	63.10	
950 -	1.273	1.069	50	53.45	
1000 -	1.554	1.414	50	70.20	
1050 -	1.065	1.310	50	65.50	
1100 -	1.722	1.394	50	69.20	
1150 -	2.698	2.210	50	110.50	
1200 -	1.943	2.320	50	116.00	
1250 -	3.805	2.874	50	143.20	
1300 -	4.380	4.092	50	204.60	
1350 -	3.071	3.725	50	186.25	
1400 -	3.310	3.196	50	159.50	
1450 -	4.550	3.930	50	196.50	
1500 -	3.470	continuation	50	200.00	

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
1500 →	3.420			B.F. →	3279.95 ³
1550	2.910	3.190	50		159.50
1600	4.862	3.736	50		186.80
1650	3.278	4.120	50		208.50
1700	4.800	3.939	50		196.95
1750	3.692	3.896	50		194.80
1800	4.386	4.039	50		201.95
1850	3.544	3.965	50		198.25
1900	3.278	3.641	50		182.05
1950	4.225	3.982	50		199.10
2000	3.622	3.924	50		196.20
2050	3.232	3.426	50		171.30

2100	1.220	2.251	50		112.55
2150	0.826	1.013	50		50.65
2200	1.510	1.133	50		56.65
2250	0.980	1.245	50		62.25
2300	1.791	1.386	50		69.30
2350	1.546	1.668	50		83.40
					⇒ 5810.15 ³

(i) from low up to 1000 m

⇒ 30% of 5810.15 m³ ⇒ 1743.05³

(ii) from initial low 1000 m

⇒ 20% of 5810.15 m³ ⇒ 1162.03³

M. S. S. S.
03.01.2020
G. S.

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
500 - CD Works 2279.95					2279.95
(5) E/W - excavation of Junction					85.12
1000 mm ϕ $\rightarrow$ 20.05					
600 mm ϕ $\rightarrow$ 21.05					
H.W. = $2 \times 20.05 \times 6.45 \text{ m} \times 1.40 \times 1.50 \text{ m} = 54.18 \text{ m}^3$					
$2 \times 21.05 \times 2.50 \text{ m} \times 1.40 \times 1.45 \text{ m} = 20.30 \text{ m}^3$					
Beddy $2 \times 20.05 \times 5.78 \text{ m} \times 1.53 \times 0.55 \text{ m} = 9.23 \text{ m}^3$					
$2 \times 21.05 \times 5.78 \text{ m} \times 1.20 \times 0.45 \text{ m} = 5.20 \text{ m}^3$					
					<u>$\Rightarrow 89.41 \text{ m}^3$</u>
(6) Pro. P.C.C. 115 - Junction					
H.W. = $2 \times 20.05 \times 2 \times 6.45 \text{ m} \times 1.40 \times 0.15 \text{ m}$					$\Rightarrow 5.418 \text{ m}^3$
$2 \times 21.05 \times 2 \times 2.50 \text{ m} \times 1.40 \times 0.15 \text{ m}$					$\Rightarrow 2.10$
Munc 17.02					<u>$\Rightarrow 7.518 \text{ m}^3$</u>
(7) Pro. 100mm Blk (1.4) - Sub-Str.					45.12
H.W. = $2 \times 20.05 \times 2 \times 6.25 \text{ m} \times 1.25 \times 0.50 \times 2.45 \text{ m}$					$\Rightarrow 53.59 \text{ m}^3$
$2 \times 21.05 \times 2 \times 2.30 \text{ m} \times 1.25 \times 0.40 \times 2.00 \text{ m}$					$\Rightarrow 68.72 \text{ m}^3$
Less for pipe's offset					
$2 \times 20.05 \times 2 \times (1.22)^2 \times 0.65 \text{ m} = 3.038 \text{ m}^3$					
$2 \times 20.05 \times 2 \times (0.82)^2 \times 0.60 \text{ m} = 1.267 \text{ m}^3$					
Parapet (100 mm)					<u>$\Rightarrow 64.469 \text{ m}^3$</u>

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(8) For type-B bedding for H.P.					
					of B.D.
1000 m ² of = 2 nos					
2 nos of 5.78 m x 1.58 m x 0.85 m = 9.73 m ³					
600 m ² of = 2 nos					
2 nos of 5.78 m x 1.00 m x 0.45 m = 5.20 m ³					
					14.93 m ³

(9) For laying Rec. NB H.P.					
of 1000 m ² of —					of B.D.
2 nos of 3 x 2.50 m = 15.00 m ³					
(10) For laying Rec. NB H.P.					
of 600 m ² of —					of B.D.

$$2 \text{ nos of } 3 \times 2.50 \text{ m} = 15.00 \text{ m}^3$$

Mun. 06.3.22

(11) constn. of C.S.B. with well					
gravel side. of I. —					
for B.T. positions					
2 x 30 m x 4.05 m x 0.20 m = 48.60 m ³					
10 x 30 m x 4.05 m x 0.20 m = 243.00 m ³					
10 x 30 m x 4.05 m x 0.20 m = 243.00 m ³					
10 x 30 m x 4.05 m x 0.20 m = 243.00 m ³					
10 x 30 m x 4.05 m x 0.20 m = 243.00 m ³					
10 x 30 m x 4.05 m x 0.20 m = 243.00 m ³					
for C.C. positions (Earthwork)					
6 x 80 m x 3.75 m x 0.10 m = 67.50 m ³					
1 x 20 m x 3.75 m x 0.10 m = 7.50 m ³					

(12) For 2 ft. typical masonry
interstory beam

2x10.5 . c/p

Area

31.7.22
0.5

30/4/22

Abstract of cost
(from Alcabid)

(1) Self pour & working
bench marks

8210

Qty indep - 0.10 ft. MB = 2x10.5

@ 4009.78/- = 80200

(2/2) Constr. of roof. pillars/
beams

1510

Qty indep - 0.10 ft. MB = 10x10.5

@ 1851.54/- = 185150

(3/3) clearing & grubbing of soil

Qty indep - 0.10 ft. MB = 0.834 ct.

@ 51133.76/100 = 424410

(4/5) constr. of earthen bank mat
with mate. from borrow

pits (low 1000 m)

Qty indep - 0.30 ft. MB = 1243.05 m³

@ 175.22/m³ = 3054170

537439300

C.O.

5/6 contn. of embankment
with mate. from borrow
pits —————
B.F. = 374393 =

Str. index - 03 of H.M.B. = 4067.10 m³

6/8 contn. of GSB with well
graded mate. G.S. —————
@ 139-85/m³ = 568784 =

Str. index - 05 of H.M.B. = 1338.60 m³

7/7 Pro. for typical
mm G.S. uniformity & 150
down —————
@ 2960-39/m³ = 3962778 =

Str. index - 06 of H.M.B. = 20105

8/22 Fill — excavation
of down —————
@ 9465-58/- = 18931 = 0

Str. index - 04 of H.M.B. = 89.41 m³

9/23 Pro. type-B' bedding for
H.F. with L.G. —————
@ 269-32/m³ = 24080 =

Str. index - 05 of H.M.B. = 14.93 m³

10/24 Pro. P.C.C. M. 15 — down
—————
@ 453-20/m³ = 6766 = 0

Str. index - 04 of H.M.B. = 7.518 m³

—————
@ 5736-10/m³ = 43124 =

Continuation

54998856 =
C.O.

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
					H.F. $\Rightarrow 4998856$
(11/65) Pro. 100 A one B/W (1:4)					
Substr.					dBW
Qty. in dep - 04 of 11 MB = 69.969m					
					@ 5839-98/m = 408618 =
(12/26) Pro. 2 figs of Rec. of 3 MB					
of 600 MB					dBW
Qty. in dep - 05 of 11 MB = 115.00m					
					@ 2583-24/m = 38749 =
(13/27) Pro. 2 figs of Rec. of 3 MB					
H.f. of 1000 MB					
Qty. in dep - 05 of 11 MB = 115.00m					
					@ 3556-82/m = 53652 =
					$\Rightarrow 5499805 =$
Add GST @ 12% of 5499805 =					659985 =
Add Lab. cess @ 1% of 5499805 =					54999 =
Add S.F. @ 1.23% of 5499805 =					69848 =
					$\Rightarrow 6284707 =$
Less 4.05% below 6284707 =					254531 =
Total value of work done					6030176
					cap
Memo:					22
31.3					0.2
					301612