

Name of work -
 Situation of work -
 Agency by which work is executed -
 Date of measurement -
 No. and date of agreement.
 (These four lines should be repeated at the commencement
 of the measurements relating to each work.)

Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
Plot on Afr Bill					
NFT work 1- Turahatti Ward NO-5					
to Lal Tal Ward NO-04					
u/t MMG Scheme					
Agreement no)- 28/ SBD/ MMG/ 20-21					
Date of survey (on) - 08/02/2020					
Date of Completion - 07/03/2020					

RECORDED ENTRY

① P/lv Cleaning works in Bawali road	= 1.40 K.M
② P/lv Retaining Pillars	= 1.40 K.M
③ P/lv cleaning and grubbing	
of Root Lash - 1 -	
40 x 30 H x 2 x 1.50 = 3600 m ²	
6 x 30 H x 2 x 1.50 = 540 m ²	
1 x 20 H x 2 x 1.50 = 60 m ²	
	4260 m ²
	Ans = 0.426 Hect.
18/02/2020	18/02/2020
J.E	A.B

Continuation

Attested.

Omrao H
 EXECUTIVE ENGINEER
 RAILWAYS DIV. ROORKEE
 19/02/21
 19/02/21

Sch. XLV-Form No. 134

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Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
① Plv Excavation substrate and earthaul					
18 x 30 m x 8.40 + 7.50 x 0.30 =	1287.90				
2					
15 x 30 m x 8.50 + 7.45 x 0.30 =	1076.625				
2					
8 x 30 m x 8.45 + 7.5 x 0.30 =	574.20				
1 x 20 m x 7.95 (Av.) x 0.30 =	47.70				
	2986.425				
② Plv and fixing of Tiffet					
M.M.95 sign Board	1				
	= 02 nos.				
Plv	200m				
soil	AB				
7.6					

R.C.C. Box Culkt)			
① Plv Earthwork excavation			
In ground as per	0		
Patt. 2 x 2.50 m x 6.0 x 1.0 m =	90.00		
Cutter			
Earth - 3 x 2 x 6.50 x 0.50 x 1.80 =	35.10		
Site clearance - 3 x 2 x 6 x 8.0 x 0.360 =	125.10		
	38.80		
	38.163.98		
② Plv earth filling in foundation			
trench as per	0		
Patt. 3 x 2.50 x 6.0 m x 0.1 m =	4.50		
Cutter			
Earth - 2 x 3 x 6.50 x 0.50 x 0.150 =	2.925		
	2.425		
③ Plv R.C.C. in H.S. in			
open foundn	0		
Patt. 2 x 2.50 x 6.0 x 0.1 m =	9.00		

Continuation

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Particulars	Details of actual measurement				Co of area
	No.	L.	B.	D.	
<u>Slab Distribution (longer)</u>					
10 mm dia @ 200 c/c					
$2 \times 11 \times 6.0 \text{ @ } 0.617 \text{ kg/cm}^2 = 81.44 \text{ kg}$					
<u>Slab Angle Bar 8 mm dia @ 100 c/c</u>					
$2 \times 40 \times 1.40 \text{ @ } 0.4 \text{ kg/cm}^2 = 44.80 \text{ kg}$					
<u>Chair 10 mm dia Net Bars</u>					
$1 \times 10 \times 1.25 \text{ @ } 0.617 \text{ kg/cm}^2 = 7.71 \text{ kg}$					
<u>Perpetual Distribution Bars</u>					
10 mm dia Bars 200 c/c					
$4 \times 4 \times 2.50 \text{ @ } 0.617 \text{ kg/cm}^2 = 24.68 \text{ kg}$					
$\text{Net Bar: } 4 \times 12 \times 1.0 \text{ @ } 0.617 \text{ kg/cm}^2 = 29.62 \text{ kg}$					

338.95 kg.

(1)

Total Reinforcement (A+B)

$$1271.89 \text{ kg} + 338.95 \text{ kg} = 1610.84 \text{ kg.}$$

Sags: 1.610 HT

$$\text{fr D.W.C. Calc: } 1.610 \times 3 \text{ M.F. } 4.83 \text{ HT}$$

Sr
05/01/2021 *BS 6119-1*
AE

(1) S.W Rec H.W.S In Brackets

$$\text{Deck: } 3 \times 4.0 \times 2.50 \times 0.250 = 11.25 \text{ m}^3$$

$$\text{Floor: } 3 \times 2 \times 6.0 \times 0.150 \times 0.150 = 0.270 \text{ m}^3$$

$$\text{Roof: } 3 \times 2 \times 2.50 \times 0.250 \times 0.400 = 0.250 \text{ m}^3$$

$$\text{R. instl: } 3 \times 4 \times 2 \times 0.250 \times 2.50 = 15.0 \text{ m}^3$$

$$29.32 \text{ m}^3$$

$$\text{A.R. at Y. end of T.H.D. P.M. - (1) I.I.E.M. (1) } (+189.21 \text{ m}) \\ \text{Continuation } 68.520 \text{ m}$$

Sr
15/01/2021 *BS 6119-1*
AB

g-E

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ABSTRACT OF COST

- (1) PIV const of Weight Bench
 max wts = 100 kg
 $1.40 \text{ K.H} \rightarrow \text{TB. P.I.D. (1)}$
 $\sigma_{P.S} = 103.65 \div 76 \text{ K.H} \quad \Rightarrow \quad \sigma_p = 14512 \div 2$

(2) PIV const of lateral pillars
 $1.40 \text{ K.H} \rightarrow \text{TB. P.I.D. (1)}$
 $\sigma_{P.S} = 10.639 \div 14 \text{ K.H} \quad \Rightarrow \quad \sigma_p = 14895 \div 2$

(3) PIV cleaning and stability
 last const = 100 kg
 $0.420 \text{ K.H} \rightarrow \text{TB. P.I.D. (1)}$
 $\sigma_{P.S} = 511.33 \div 76 \text{ K.H} \quad \Rightarrow \quad \sigma_p = 21476 \div 2$

(4) PIV constn. of Embankments
 refact form = 100 kg

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Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
Load upto 10m m					
359.10 M ³ -> T.H.B. P. N.O. - (6)					
@ P = 174 = 94 / M ³				P = 62,821 :-	
(1) Plv Cont of Sub ground water					
Bottom & Loulda - to					
2986.425 M ³ -> T.H.B. P. N.O. - (2)					
@ P = 1762.58 / M ³				P = 5,273.43 :-	
(2) Plv Cont of Bimbawat					
Water from 10m m					
851.40 M ³ -> T.H.B. P. N.O. - (6)					
@ P = 1392.84 / M ³				P = 1,190.60 :-	
(3) Plv Cont of S.I.B by well					
ground water - to					
97.20 M ³ -> T.H.B. P. N.O. - (6)					
@ P = 2494.9.33 / M ³				P = 2,867.23 :-	
(4) Plv Bldt hole excavation					
In found at bld					
163.98 M ³ -> T.H.B. P. N.O. - (1)					
@ P = 269.82 / M ³				P = 44,165.2 :-	
(5) Plv Cont filling in ground					
trenches as bld					
7.425 M ³ -> T.H.B. P. N.O. - (2)					
Limit = 7.92 M ³					
@ P = 40.49 / M ³				P = 293.2 :-	

Continuation

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Particulars	Details of actual measurement				Contents of area
	No.	L.	B.	D.	
(16) 34) P.W. P.C.C. M.I.S. f. open fourth as per - to -					
11.925 ft. \times T.H.B. P. H.O. - (3)					
Limit = 11.34 ft					
$eP = 5938 \times 42 / 10 = P = 67342 \text{ ft}^2$					
(17) 35) P.W. Acre-c M.I.S. grade in Bam. call as per - to -					
68.520 ft. \times T.H.B. P. H.O. - (3)					
Limit = 68.13 ft					
$eP = 7437.15 / 10 = P = 506720 \text{ ft}^2$					
(18) 36) P.W. Bam. Colly. Bal. P. Add. P. well - to -					
49.20 ft. \times T.H.B. P. H.O. - (6)					
$eP = 2723 \times 48 / 10 = P = 120246 \text{ ft}^2$					
(19) 37) P.W. S. B.A.Y. G. H. f. and Placing H.Y.S.D. Bam. - to -					
4.83 ft. \times T.H.B. P. H.O. - (3)					
$eP = 45.933 \times 78 / 10 = P = 2,21860 \text{ ft}^2$					
(20) 38) P.W. a. & A. C. f. H.M.S.C. Sagri Bam. - to -					
02 NO. \times T.H.B. P. H.O. - (2)					
$eP = 11.45 \times 50 / 10 = P = 22919 \text{ ft}^2$					
$P = 2033,012 \text{ ft}^2$					
Add 12% G. S. T. $P = 2,43,961 \text{ ft}^2$					
Add 12% I. Corr. $P = 20330 \text{ ft}^2$					
Continuation $P = 22,97,303 \text{ ft}^2$					

(C)

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J.E

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100

PREF

Conclusion

Attested

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